

TABLE TO § 3285.312—THE SIZE AND CAPACITY FOR UNREINFORCED CAST-IN-PLACE FOOTINGS—
Continued

Soil capacity (psf)	Minimum footing size (in.)	8 in. × 16 in. pier		16 in. × 16 in. pier	
		Maximum footing capacity (lbs.)	Unreinforced cast- in-place minimum thickness (in.)	Maximum footing capacity (lbs.)	Unreinforced cast- in-place minimum thickness (in.)
2,500	16 × 16	4,300	6	4,300	6
	20 × 20	6,700	6	6,700	6
	24 × 24	⁴ 9,600	8	9,700	6
	30 × 30	⁴ 14,800	10	15,000	8
	36 × 36	⁴ 20,700	12	⁴ 21,400	10
3,000	16 × 16	5,200	6	5,200	6
	20 × 20	8,100	8	8,100	6
	24 × 24	⁴ 11,500	10	11,700	6
	30 × 30	⁴ 17,800	12	⁴ 18,100	8
	36 × 36	⁴ 25,400	14	⁴ 25,900	10
4,000	16 × 16	7,000	6	7,000	6
	20 × 20	⁴ 10,800	8	10,900	6
	24 × 24	⁴ 15,500	10	15,600	8
	30 × 30	⁴ 23,300	12	⁴ 24,200	10

NOTES: 1. The footing sizes shown are for square pads and are based on the area (in.²), shear and bending required for the loads shown. Other configurations, such as rectangular or circular configurations, can be used, provided the area and depth is equal to or greater than the area and depth of the square footing shown in the table, and the distance from the edge of the pier to the edge of the footing is not less than the thickness of the footing.

2. The 6 in. cast-in-place values can be used for 4 in. unreinforced precast concrete footings.

3. The capacity values listed have been reduced by the dead load of the concrete footing.

4. Concrete block piers must not exceed their design capacity of 8,000 lbs. for 8" × 16" single stack block and 16,000 lbs. for 16" × 16" double stack block.

5. A registered professional engineer or registered architect must prepare the design, if the design loads exceed the capacity for single or double stack concrete block piers shown in footnote 4.

§ 3285.313 Combination systems.

Support systems that combine both load-bearing capacity and uplift resistance must also be sized and designed for all applicable design loads.

§ 3285.314 [Reserved]

§ 3285.315 Special snow load conditions.

(a) *General.* Foundations for homes designed for and located in areas with roof live loads greater than 40 psf must be designed by the manufacturer for

the special snow load conditions, in accordance with acceptable engineering practice. Where site or other conditions prohibit the use of the manufacturer's instructions, a registered professional engineer or registered architect must design the foundation for the special snow load conditions.

(b) *Ramadas.* Ramadas may be used in areas with roof live loads greater than 40 psf. Ramadas are to be self-supporting, except that any connection to the home must be for weatherproofing only.

Subpart E—Anchorage Against Wind

§ 3285.401 Anchoring instructions.

(a) After blocking and leveling, the manufactured home must be secured against the wind by use of anchor assembly type installations or by connecting the home to an alternative foundation system. See § 3285.301.

(b) For anchor assembly type installations, the installation instructions must require the home to be secured against the wind, as described in this section. The installation instructions and design for anchor type assemblies must be prepared by a registered professional engineer or registered architect, in accordance with acceptable engineering practice, the design loads of the MHCSS, and § 3285.301(d).

(c) All anchoring and foundation systems must be capable of meeting the